

Column size $\rightarrow$ W10x33

Pin-Pin column $\rightarrow k_y = 1.0$

$L = 10\text{ft} \rightarrow L_x = 20', L_y = 20'$

Along x-axis

Effective length $\rightarrow (k_x) \cdot (L_x)$

$$1 \times 10' = 10'$$

Along y-axis

Effective length $\rightarrow (k_y)(L_y)$

$$(1) \times (10') = 10'$$

Column chosen: W10x33

$$E = 29000 \text{ ksi}$$

$$I_x = 171 \text{ in}^4$$

$$I_y = 30.6 \text{ in}^4$$

Buckling strength:

x-axis:

$$P_{cr-x} = \frac{\pi^2 E I_x}{(k_x L_x)^2}$$

$$= \frac{\pi^2 \times 29000 \times 171 \text{ in}^4}{(1 \times 10' \times 12)^2} = 3398.85 \text{ kips}$$

y-axis:

$$P_{cr-y} = \frac{\pi^2 \cdot E \cdot I_y}{(k_y \cdot L_y)^2}$$

$$= \frac{\pi^2 \times 29000 \times 30.6}{(1 \times 10' \times 12)^2} = 727.47 \text{ kips}$$

Effective lengths:

$$r_x = 4.19 \text{ in}$$

$$r_y = 1.94 \text{ in}$$

$$\rightarrow \frac{k_x L_x}{r_x} = \frac{1 \times 10' \times 12 \text{ in}}{4.19} = 28.639$$

$$\rightarrow \frac{k_y L_y}{r_y} = \frac{1 \times 10' \times 12}{1.94} = 61.86 \rightarrow \text{Slenderness ratio governs}$$

Column strength.

$$\frac{K_y \cdot L_y}{r_x} = \frac{10'}{\frac{1.44}{4.19}} = 21.60 \text{ ft.}$$

Factored design load:

$$\begin{aligned} P_u &= 1.2(D) + 1.6(L) \\ P_u &= 1.2(150) + 1.6(50) \\ &= 260 \end{aligned}$$

$$\begin{aligned} D &= 150 \\ L &= 50 \end{aligned}$$

Column Design

Second level

A-2

$$DL = 20 \quad LL = 40$$

$K=1 \rightarrow$ Column (Pinned - Pinned)

Col W10x33

$$I_x = 171 \rightarrow AISC 14th$$

$$I_y = 36.6 \quad r_y = 1.94 \quad f_y = 50$$

$$r_x/r_y = 2.16$$

$$\frac{KL}{r_y} = \frac{1 \times 12 \times 10}{1.94} = 61.85$$

$$4.71 \sqrt{\frac{E}{f_y}} = 4.71 \sqrt{\frac{29000}{50}} = 113.43$$

$$\frac{KL}{r_y} < 4.71 \sqrt{\frac{E}{f_y}}$$

$$F_{cr} = 0.658^{\frac{f_y}{F_{cr}}} f_y$$

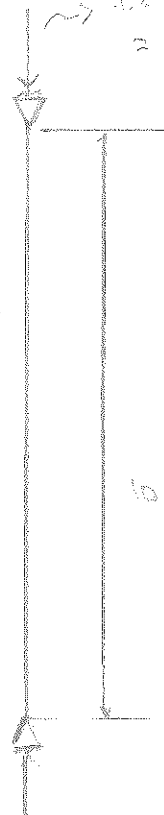
$$= 0.658^{\frac{50}{37.8}} \times 50$$

$$= 37.8$$

$$\phi P_n = 0.9 F_{cr} \times A$$

$$= 0.9 \times 37.8 \times 37.8 = 330^k > 176^k \text{ OK}$$

$$\sim 1.2 \times 20 \times 2 + 1.6 \times 40 \times 2 = 176^k$$



$$P_c = \frac{\pi^2 E}{61.85^2} = 74.82$$

$$A = 9.71$$

Column Design

Second Floor

7-B

$$K=1$$

$$r_g = 4.44$$

$$\frac{K L}{r_g} = \frac{27.081}{4.44} = 61.85$$

W10X33

$$\lambda_c = \sqrt{\frac{E}{F_y}} = 113.43$$

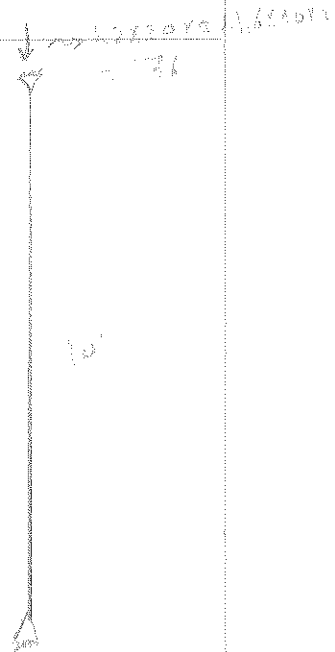
$$4.71 \sqrt{\frac{E}{F_y}} > \frac{K L}{r_g}$$

$$F_e = \frac{\pi^2 \times 29000}{61.85^2} = 39.82$$

$$F_{cr} = 0.658^{\frac{F_y}{F_e}} \times F_y = 32.8$$

$$\phi P_n = 0.9 \times F_{cr} \times A$$

$$= 0.9 \times 32.8 \times 38.7 = 1147.6 \text{ OK}$$



Column Design

First floor

A-2

$$DL = 20 \quad LL = 40$$

$$K = 1$$

W10x33

$$r_y = 1.94$$

$$\frac{K L}{r_y} = \frac{12 \times 12}{1.94} = 74.22$$

$$4.71 \sqrt{\frac{E}{F_y}} > \frac{K L}{r_y}$$

$$\downarrow$$

$$113.43$$

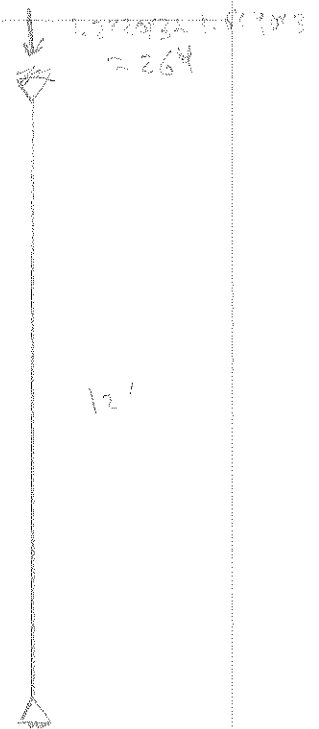
$$F_{cr} = 0.658 \frac{F_y}{\left(\frac{K L}{r_y}\right)^2}$$

$$= 33.47$$

$$A = 9.71$$

$$\phi P_n = 0.9 F_{cr} A = 0.9 (33.47) (9.71)$$

$$= 297 \text{ k} > 264 \text{ k} \quad \text{OK}$$



Column Design

First Floor

7-8

$$DL = 20 \quad LL = 40$$

$$K = 1$$

$$r_y = 1.94$$

W10x33

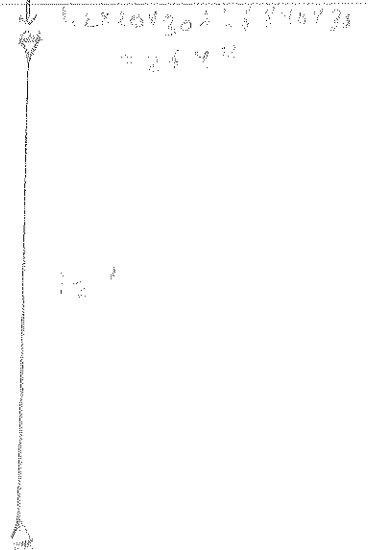
$$\frac{KL}{r} = \frac{12 \times 12}{1.94} = 74.22$$

$$\lambda_c = \sqrt{\frac{F_y}{F_c}} = 113.43$$

$$F_c = 0.658^{\frac{F_y}{F_c}} \times F_y$$

$$F_c = \frac{50 \times 29000}{74.22^2} = 27.35$$

$$\phi K_n = 0.9 \times F_c \times A = 0.9 \times 27.35 \times 33 \times 42 = 2915 > 2704 \quad \text{OK}$$



Column Design

Third Floor

4-C

$$DL = 20 \quad LL = 40$$

$$K = 1$$

$$W10 \times 33$$

$$K_y = 1.94$$

$$\frac{K_y L}{r_y} = \frac{1.94 \times 10}{1.94} = 10.0$$

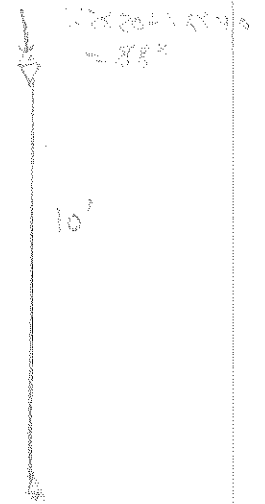
$$1.7 \sqrt{\frac{E}{F_y}} = 1.7 \sqrt{\frac{29,000}{50}} = 48.4$$

$$\frac{K_y L}{r_y} < 4.71 \sqrt{\frac{E}{F_y}}$$

$$F_{cr} = 0.658^{\frac{F_y}{F_{cr}}} \times 50 = 37.8$$

$$A = 31.1$$

$$\phi P_n = 0.9 \times F_{cr} \times A = 0.9 \times 37.8 \times 31.1 = 330 > 80$$



$$F_{cr} = \frac{\pi^2 E}{(K_y L)^2} = 37.8$$

Column Design

Third floor

7-8

DL-20 DL-25

$$K = 1$$

W 10432

117X5-40
1.94
SS. 174

Temp. at 3 ¹⁰ 40 = 40

Se $\frac{1}{2} \times 72000 = 36000$ 74

$$R_p = 0.4 \times 40 \times 0.75 = 12 \text{ kN} > 8.8 \text{ kN}$$

