

Project: SWB

Job No.

Sheet: of

Item: Bent 2

Designer: MGL

Date: 5/12

Checker:

Date:

Grid: 1/10"

Base of wall at Bent 2 (joint)

1000-year no-scour case "B":

$$V = 3265^k, P = 27,605^k, M = 181,864^{ft-k}$$

w/1.2:  $V = 3918^k \quad M = 218,237^{ft-k}$

- Vertical (gravity) load uniformly distributed at base of wall.

Area of base:  $A = 2(26 \times 9.92) + 33 \times 4 = 647.8 \text{ ft}^2$

- Assume that the longitudinal moment and shear are resisted entirely by the wall "risers". Vertical load tributary to each riser:

$$P_c = 27605 \times \frac{26 \times 9.92}{647.8} = 10,990^k$$

$$M = 109,118^{ft-k}$$

$$V = 1959^k$$

- Take cap "width"  $B_{cap} = 30'$  in the longitudinal direction

$$T_c \approx \frac{109,118}{9'} = 12,124^k$$

$$A_{jv} = l_{ac} B_{cap} = (12 \times 12)(30 \times 12) = 51,840 \text{ in}^2$$

$$f_{jv} = \frac{12,124}{51,840} = 0.234 \text{ Ksi}$$

$$A_{jh} = (9 + 12) \times (30 \times 144) = 90,720 \text{ in}^2 \quad (\text{Limited by footing width: } 20 \times 30 \times 144 = 86,400)$$

↑  
embedment of bars

$$f_v = \frac{10,990}{86,400} = 0.127 \text{ Ksi}$$

$$f_h = \frac{1959}{(30 \times 12 \times 144)} = 0.038 \text{ Ksi}$$

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Principal tension:

$$P_t = \left| \left( \frac{.038 + .127}{2} \right) - \sqrt{\left( \frac{.038 - .127}{2} \right)^2 + (.234)^2} \right| = 0.166 \text{ Ksi} \quad \underline{OK}$$

Principal compression:

$$P_c = \left( \frac{.127 + .038}{2} \right) + \sqrt{\left( \frac{.038 - .127}{2} \right)^2 + (.234)^2} = 0.321 \text{ Ksi} \quad \underline{OK}$$

Principal stresses are less than  $0.11\sqrt{f'_c}$ . Case A in the half-screw condition has a smaller moment, but also a reduced axial load. Conservatively use that smaller axial load with the moment and shear from above and check again:

$$P_c = 16208 \times \frac{26 \times 9.92}{647.8} = 6453 \text{ lb}$$

$$f_v = \frac{6453}{86400} = 0.075 \text{ Ksi}$$

Principal tension:

$$P_t = \left| \left( \frac{.038 + .075}{2} \right) - \sqrt{\left( \frac{.038 - .075}{2} \right)^2 + (.234)^2} \right| = 0.178 \text{ Ksi} \quad \underline{OK}$$

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Also consider 2 cases of orthogonality:

$$\textcircled{1} P = 16,350^k; M_{\text{long}} = 116028^{\text{ft-k}}; M_{\text{trans.}} = 116029^{\text{ft-k}}$$

$$\textcircled{2} P = 27,605^k; M_{\text{long}} = 181864^{\text{ft-k}}; M_{\text{trans.}} = 131614^{\text{ft-k}}$$

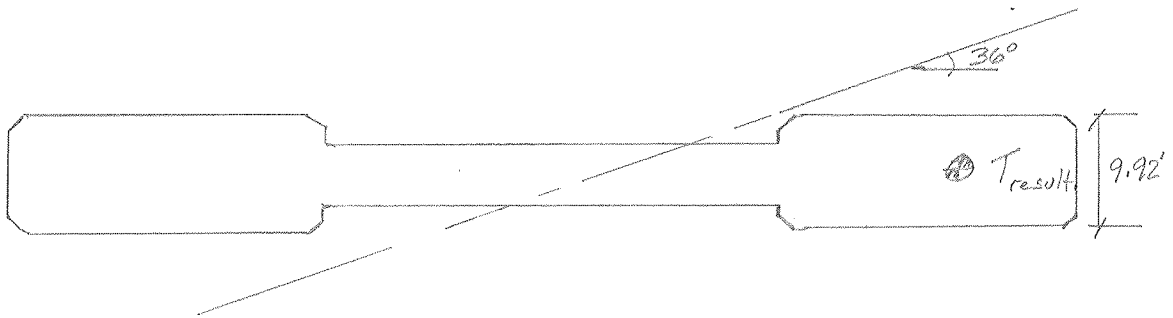
- Put these combinations into a moment-curvature analysis to determine tensile resultants to use for shear stress calcs.

Xtract output reports total tensile forces of:

$$\textcircled{1} 6702^k$$

$$\textcircled{2} 9726^k$$

$$\theta = \tan^{-1}\left(\frac{131614}{181864}\right) = 36^\circ$$



Assume that the "B<sub>cap</sub>" term is aligned with this angle and is:

$$B_{\text{cap}} = \frac{9.92'}{\sin 36} = 16.9'$$

$$A_{jv} = (12 \times 12)(16.9 \times 12) = 29,203 \text{ in}^2$$

$$V_{jv} = \frac{9726}{29203} = 0.333 \text{ Ksi}$$

→ Joint shear steel is required

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Transverse (volumetric) steel ratio required:

$$\rho_s = 0.40 \frac{A_{st}}{l_{ac}^2}$$

There are (134) #11 bars in each wall riser:  $A_{st} = 209 \text{ in}^2$

$$\rho_s = 0.40 \frac{(209)}{(12 \times 12)^2} = 0.004$$

If the (24) #5 in the wall's weak direction and the (7) #5 in the strong direction are continued into the footing:

$$\rho_s = \rho_x + \rho_y = \frac{(24 \times .31)}{(26 \times 12)S} + \frac{(7 \times .31)}{(9.92 \times 12)S} = .004$$

$$S = 10.5''$$

If the "hoops" are continued into the footing as #5 bars:

$$\rho_s = \frac{(26 \times .31)}{(26 \times 12)S} + \frac{(9 \times .31)}{(9.92 \times 12)S} = .004$$

$$S = 12.3''$$

→ Continue confinement steel into footing at 12" spacing

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Vertical steel required:

$$A_s^{jv} = 0.20A_{st} = .2(209) = 41.8 \text{ in}^2$$

- Provide at least (42) #9 headed bars on each side of each riser. Steel to be placed within 9.9' of  $\pm$  Bent 2 in each direction

Horizontal steel required:

$$A_s^{jh} = 0.10A_{st} = .1(209) = 20.9 \text{ in}^2$$

- The horizontal steel shown consists of groups of 3 #5 bars placed at 1'-0" vertical spacing, with 10 such vertical placements called for.

$$A_{s, \text{provided}}^{jh} = 3 \times 3 \times .31 \times 10 \text{ ples.} = 27.9 \text{ in}^2 \quad \underline{OK}$$

↑  
3 "clusters" of 3 bars under each riser

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- Longitudinal bending demands at the base of Bent 3 are smaller than those at Bent 2. Vertical loading and wall geometry are similar.

- Because joint shear reinforcing requirements are prescriptive, and not force-based, it is prudent to provide the same joint shear reinforcing at Bent 3 as at the more heavily loaded Bent 2 (which also has a few more #11 bars in the "Ast" term).

- Provide same joint shear detailing in both places